

April 4, 2022

City & County of San Francisco
Board of Supervisors
City Hall, Room 250
San Francisco, CA 94102

Attention: Shamann Walton, President

Subject: Millennium Tower
301 Mission Street, San Francisco
Serious Situation Requiring Attention

Dear President Walton and Board Members:

On 7/10/19, almost three years ago, the undersigned, as requested, submitted a report to the Board opining on two proposed foundation plans: Partial external asymmetrical ("SGH") and full internal symmetrical ("LERA") engineered separately for retrofitting support to the differentially settled and tilted Millennium Tower. Our report (copy attached) and this supplement are provided as a public service. The full internal symmetrical plan was "vastly superior" to the partial external asymmetrical plan, however, unfortunately, the superior plan and its well planned implementation were not selected. The condominium owners elected to be paid cash in lieu of implementing the superior internal plan.

Project Situation

Preface to the 7/10/19 report describes and compares the design and construction aspects of both of the formerly proposed Plans. The report includes predictions, the partial external plan was selected, it is likely the settling and tilting will worsen during construction, which has happened. The full internal plan would have started with and resulted in the building being supported internally with built-in mechanisms to level and plumb the building to the benefit of the condominium ownership, the City, and maintenance of the reputation of the engineering profession in San Francisco and California.

Stellar qualifications and expertise exist for LERA (structural engineering firm for the World Trade Center) and ENGEO (geotechnical engineering for extensive and successful local engineering projects) for design of the symmetrical internal plan in comparison with the questionable experience and uncertain expertise of the designers for the asymmetrical partial external plan. Adverse aspects of the partial external plan were summarized on 7/10/19 as [A] the new partial perimeter piles were much too close to each other in violation of basic geotechnical principles due to ground disturbance, interference, and not complying with 2019 SFBC §1810.2.5; [B] installing closely spaced casing and piles means "lost ground" and groundwater depletion that currently provides a measure of buoyancy; loss will increase effective stress on subgrades and substructure as the phreatic surface is lowered; [C] seismic response for the asymmetrical plan, which will be irregular due to the loss of symmetry as well as violative of 2019 SFBC §403.9.4; and [D] excavation for the pile cap in the streets along the building will decrease lateral and subjacent support at the building's base causing more loss of groundwater, vibrations, settling and tilting, and possible collapse during the long process of construction. This report focuses on pending excavation into existing lateral and subjacent support, which could be fatal.

Construction & Review

Project construction records are in the form of periodic monitoring reports containing engineering data (and disclaimers), as of 3/22/22 numbering 46 documents, provided to the City Building Department (SBDBI) by civil engineers using a company titled Slate Geotechnical Consultants. Their first report, 001 (5/7/21) shows, graphically, lateral roof deflection moving from plumb (1/1/10) to 18 inches (5/1/21) in 149 months (0.121 inches/month). Slate's latest report, 046 (3/22/22), shows lateral deflection moving from 0 inches (1/1/21) to 8.4 inches (3/12/22) in 14.4 months (0.583 inches/month). Slate's monitoring data indicates lateral deflection increased 408% faster after construction began than before.

Alarmed by the increase in lateral roof deflection (tilt, a function of the building's differential settlement), the building's trajectory, mainly toward the Salesforce Transit Center Plaza and Salesforce Tower, the City called for and held two hearings, 11/4/21 and 1/6/22, where several involved people testified; most critical was the Project sponsor's designer and the City's Chairman for engineering and construction review. Although the principal designer for the "Perimeter Pile Upgrade" testified at the first hearing that the building could tilt 79 inches (6'-7") without danger and ridiculed critics of his theories justifying his design, the most notable testimony in the transcript was that of Gregory Deierlein, chairman of the Engineering Design Review Team (EDRT) who confirmed safe tilt of 79 inches (pg 177); when asked if the Team had analyzed during-construction effects (settlement/tilting) before construction work started; replied that had not been done, which was termed "a little weird" by Supervisor Peskin (pg 174).

In that construction engineering for deep foundations is the most important aspect of construction of major buildings in Yerba Buena Cove, the information that only theoretical engineering using computer analyses with input of assumptions was used without studying ways and means of construction by experienced geotechnical engineers and constructors, was astounding information. As it turns out, the Chairman of EDRT was a political appointee and does not happen to be a licensed engineer. In California, a state profoundly affected by earthquakes where civil engineer applicants are so tested, registration (civil engineer license) is [minimum] evidence of qualification (B&P Code §6730). It was found by researchers (not the undersigned) the only authoritative credentials issued to the Chairman by California are a driver's license and Incorporation (C4144317, 3/31/19) to limit personal liability.

Important Focus

This correspondence is not intended to be a critique of the design further to the discussion in the 7/10/19 report (e.g. page 3 paragraph 4) of the partial external plan which creates plan irregularity—contrary to the claim that the asymmetrical plan is a "voluntary seismic upgrade" of the building. Nor is this correspondence a detailed discussion about the troubles due to construction of the selected design by SGH. The record of what has happened speaks for itself (which cannot be corrected unless the project sponsors see the light and return to the vastly superior LERA design) as the monitoring discussed above gives data indicative of an inappropriate design and failure to control construction that has worsened settlement and tilting which were predicted by the undersigned.

Significantly, an independent engineering firm, Dan Brown and Associates, in a review requested by the City, observed (report 11/23/21) an installation of a casing and pile then finding from concrete delivery records that enough ground was lost during construction to require about 5 cubic yards of concrete to make up the loss. This result, which was made public, resulted in changing the design from 52 piles (to 42 piles) to 18 piles without explanation as to why 52 piles were ever shown on drawings submitted to the City with the Permit Application for the retrofit project. All of a sudden 65% of the piles were simply not needed without any acknowledgment that the permitted design showed piles that were much too closely spaced as prohibited by Code and predicted by fundamental geotechnical engineering principles as noted in the undersigned's 7/10/19 report and applicable to deep foundations in weak subsoil conditions.

The problem at hand, allowing excavation along the already compromised west side of the building, is very serious and could lead to toppling of the building during an earthquake. Any risk of building failure is unacceptable. The consequences of the building falling onto the transit plaza that thousands of commuters use to travel to and from the East Bay is unimaginable even though possible. The ongoing and increasing tilt is worsening, not slowing, due to the work. Excavating along the western and northern building edges began, fittingly, on April 1. Excavating will remove the left-in-place original shoring wall for construction of the existing 25 foot deep basement thereby removing lateral and subjacent support for the leading building edges where differential settlement causing the tilt has occurred and will continue to occur. During strong motion generated by earthquakes, the lost lateral and subjacent support could cause toppling of the building as the restraint from the existing prestressed concrete piles is problematical to say the least.

Before proceeding with further excavation, the City must require physical investigations and independent analysis of the structural resistance of the existing prestressed concrete piles (7/10/19 report, pg 3 ¶4) as well as any existing external construction and/or connections may provide to resist overturning. The safety reviews (7/29/17 and 8/27/19) by EDRT used information provided by the designer, a surficial inspection, assumptions, hearsay, and conjecture that cannot be relied upon for serious matters. For instance, is there a "seismic gap" between buildings? (7/29/17 pg 7) or are the buildings connected as "extensive concrete cracking and spalling [has occurred] in the basement walls of the neighboring 12-story Mid-rise building."

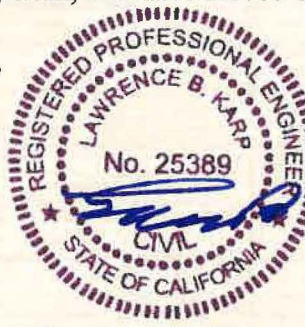
More recently, responding to criticism of creating plan irregularity namely the sufficiency of the heads of the existing prestressed concrete piles, the Chairman declared the pile reinforcing into the tilted basement slab foundation "will act as springs" during an earthquake which is unlikely as the pile connections of the 14 inch square piles to the 10 foot thick mat are likely damaged by rotation (7/10/19 report, pg 3 ¶4) and tension. EDRT does not acknowledge LERA's analyses (e.g. rotation of the heads of existing piles).

The required physical investigation should have been done long ago but now is imperative, regardless of protests. Investigation of a typical pile to foundation mat connection can be accomplished by cutting away between closely spaced cores at a location on the east side of the basement. Properly done, this physical investigation will determine if the piles have been damaged by combined stress from rotation due to tilting of the mat foundation slab or will the piles, especially those distant from the tilting edge of the building, serve to restrain the building from increased movement due to excavating away support on Fremont Street.

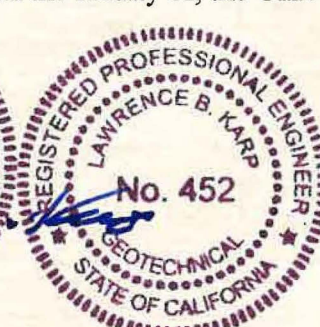
Disclosures

Neither of the undersigned have any financial or business or other affiliation with any phase of the Millennium Project or with any developer or condominium owner or any design professional or any contractor. However, each of the undersigned has over 50 years experience in evaluation, design, and construction of buildings and other structures and their foundations in San Francisco, the Bay Area, and elsewhere. Each of the undersigned holds multiple degrees including earned doctorates in Civil Engineering from, and have served on the faculty of, the University of California at Berkeley.

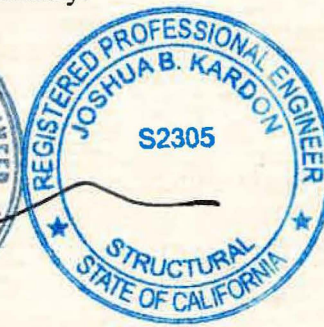
Yours truly,



Lawrence B. Karp



Joshua B. Kardon



cc: Neville Pereria CE, C&CSF Deputy Director of Permit Services

LAWRENCE B. KARP
CONSULTING GEOTECHNICAL ENGINEER

FOUNDATIONS, WALLS, PILES
UNDERPINNING, TIEBACKS
DEEP RETAINED EXCAVATIONS
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SOIL MECHANICS, GEOLOGY
GROUNDWATER HYDROLOGY
CONCRETE TECHNOLOGY

July 10, 2019

City & County of San Francisco
Board of Supervisors
Legislative Chamber
City Hall, Room 250
San Francisco, CA 94102

Attention: Norman Yee, President

Subject: Millennium Tower
301 Mission Street, San Francisco
Two Differing Plans for Foundation Retrofitting

Dear President Yee and Board Members:

This correspondence briefly discusses two foundation plans (external asymmetrical and internal symmetrical) engineered separately for retrofitting support to the subsided and tilted Millennium Tower. This report is provided as a public service as one of the plans seriously affects the City.

Preface

The External Asymmetrical Plan enlarges the footprint of the building by utilizing portions of Fremont and Mission streets for shoring, trenching, drilling and casing new foundation piles to support extensions of the building's substructure along part of its outside perimeter to prop the lower half of the building, which result is projected to support about one-fifth of the building's total load. The Internal Symmetrical Plan consists of jacking new piles arranged in a systematic pattern inside the building to provide new uniform support for the entire building. Piles into bedrock are used for foundation support in both plans. The final cost estimate for the completed refined internal plan is estimated to be less than the cost projected for the preliminary external plan. The cost differential will increase because the external plan requires ownership or easement use of City property in the streets, involves traffic disruptions and expenses, and because of probable damages due to dewatering and lost ground; these and other undeterminable but very significant additional direct and indirect construction costs and delays will ultimately be added as contingencies to the cost of the external plan. The internal plan is isolated and self contained within the building envelope without any participation by the City. This correspondence summarily covers, by pros and cons associated with major factors, why the internal plan is vastly superior to the external plan.

Building

The Millennium Tower is a 58 story concrete framed building built 8/06-2/08, 645 feet high, housing 366 residences. The basement is 25 feet deep incorporating a 10 foot thick reinforced concrete mat with 920 prestressed concrete production piles, driven 47 to 83 feet deep through heterogeneous fill (the building is situated at the 1852 shoreline) and marine deposits such that their tips were founded in dense sands. The Tower is said to be the highest and heaviest concrete building in the western US.

Causation

The Millennium Tower subsided due to the extraordinary overall building loads imposed upon the subgrade, and settled more differentially toward the corner of Mission and Fremont Streets due to the consequences of groundwater pumping which lowered the phreatic surface (groundwater level) along the northwest corner of the building. Dewatering from buildings along those streets added loads to the Millennium Tower's subgrade due to loss of buoyancy by the water and increase in effective vertical stress in the subgrade. Loss of water content also causes silts and clays to compress and the additional weight of overburden soils causes clays to consolidate.

The top of the building tilts about 18 inches toward west northwest due to differential settlement of the building, which aggravates the settlement near the corner of Fremont and Mission Streets by movement of the center of gravity of the building. The differential loading and loss of support from dewatering, the greatest value where drawdown approaches the intersection of Mission and Fremont Streets, if proportioned flat from the building's corner on Minna Street almost midway to Beale Street, from the building's height, is 4.8 inches, but the mat is dished so the actual level differential near the intersection of Fremont and Mission Streets is about 3 inches.

The corner opposite from the Fremont and Mission corner of the building, on Minna, was not dewatered directly due to a 60 foot deep cut-off wall between the properties constructed by the Transbay Transit Center from outside Beale then along Minna to Fremont. By projecting available piezometric levels and slope of the building's base as an indicator of drawdown, there is a hydraulic gradient of the phreatic surface, which cannot be measured directly due to interference, sloping down toward the intersection of Fremont and Mission Streets. Both the excessive load of the concrete building supported by an inadequate pile system, and pumping from other construction and buildings along Mission Street that also drew down groundwater from Fremont Street, caused the subsidence and tilting of the building. The concrete substructure is cracked and broken from differential settlement and tilting of the building, which has severely compromised the heads of the existing thin (14 inch square) prestressed concrete piles. Dewatering along Mission Street will continue, and will probably worsen as Oceanwide Center nears completion of construction dewatering, therefore settlement of the building will continue particularly if the external plan is implemented. As deep foundation construction technology was available when the Millennium Tower was designed and built, the settlements due to the loads on the inadequate pile foundation and the loads due to dewatering that caused the tilting could have been prevented by constructing a foundation with piles reaching bedrock.

External Asymmetrical Plan

The external plan (attached schematic), designed by Simpson Gumpertz & Heger (SGH) Consulting Engineers has a total of 52 drilled and cased very closely spaced piles along Mission and Fremont Streets; 30 along Fremont and 22 along Mission that are connected by a continuous concrete pile cap that will be doweled into the existing substructure so the new piles will provide bedrock support, which will be eccentric, for about one-fifth of the building's total load.

An SPTC wall 47 feet deep will be built to shore a 27 foot deep, 10 foot wide excavations in both streets between the shoring and the existing substructure. A 5 foot thick grout plug will be constructed at the bottom of the excavation to prop the shoring at excavation subgrade, restrict backslope subsidence and basal heave, and provide an area to work in the dry. Piles will be drilled from the street and cased from bedrock to the level of the mat's bottom.

Groundwater Table Drawdown

The major initial problem with construction of the external plan that has no realistic practical or theoretical solution is further loss of groundwater, which is likely to cause more irreparable damage to the building's substructure regardless of subsequent attempts to counter the effect of supporting the existing mat with flat jacks on the piles. Drilling, casing, and removal of spoils from the site to construct closely spaced 52 piles, 36 inch diameter holes to the top of the Old Bay Mud (80 feet below street grade) to become 24 inch diameter piles more than 200 feet deep will cause loss of ground which comes with loss of groundwater that will worsen the problem from dewatering that created the condition in the first place in the area of the planned new piles.

Exacerbating the loss of groundwater with removal from Fremont and Mission Streets of about 1,500 CY of soil and water from the combined locations of the piles will be dewatering necessary to excavate the planned 10 foot wide 27 foot deep trench, 125 feet along Mission Street and 175 feet along Fremont Street, about 3,000 CY. Present groundwater levels near the intersection of Fremont and Mission Streets are at about 23 feet from street grade. With construction of the grout plug in Young Bay Mud, the trench will require groundwater levels along Fremont and Mission Streets to be drawn down to at least 33 feet from street grade, or about 10 feet down from the present GWT.

Groundwater withdrawal without recharge lowers the groundwater table which with building loads increases overburden pressure on compressible soils. Lowering the GWT another 10 feet for the design and to create workable space in the Young Bay Mud for the grout plug will cause the building to further settle differentially with associated damages to the building's substructure. From geometry, for every new inch of differential settlement of the Millennium Tower at Fremont and Mission Streets, the top of Millennium Tower will lean about an additional 4 inches. The tilt will be unacceptable to the owners of the residences and the public. Severe increased breaking and cracking of the concrete base and rotation/failure of the pile heads due to more differential settlement will cause further irreparable damage.

Plan Configuration Irregularity

The asymmetrical external foundation plan, consisting of perimeter piles along two adjacent sides of the building, will create a major horizontal structural irregularity. This will be a torsional irregularity as the new piles are relatively strong load-resisting elements but there are no comparables on the opposite sides of the building. Earthquakes have repeatedly shown that buildings that have irregular configurations suffer greater damage; for irregular plans, torsion is caused by eccentricity between the centers of mass and rigidity. In regular structures inelastic response with energy dissipation and damage produced by strong ground shaking will be distributed throughout the building. For the external foundation plan, where a seismic event will generate base shears that are derivatives of the building's weight in precisely the direction of the new pile rows which may passively resist them, forces in other directions will cause torsion to other building elements or will not be resisted by available passive, such as along Minna Street. In such irregular structures, inelastic behavior may be concentrated. The existing 14 inch square (with chamfered corners) piles, even with tops reinforced likely have rotated heads, crushed portions of concrete, and bent bars as a result of the differential settlement of the mat foundation that was meant to tie the piles together for group action. Narrow concrete piles that are prefabricated with prestressing strands are for the purpose of resisting breakage during lifting, and with cushions to resist driving forces; their heads are not meant to rotate. Ground motions acting on a skew with respect to the building's axis can cause torsion. 2016 California Existing Building Code §403.9.4, for voluntary seismic improvements, requires that alterations do not create a structural irregularity.

City Disruption & Expense

If the project is approved by the City, there seem to be questions about easement or ownership of the land that the City may grant to the Millennium Tower Association. The tiebacks for the SPTC wall will have to remain in the street area, as have the tiebacks for 301 and 350 Mission. The SPTC shoring wall, building extension, piles, and vaults will be permanent.

For the excavation into San Francisco public streets to enlarge the footprint of the Millennium Tower, 10 feet wide between the shoring wall and 300 feet long (125 feet along Mission and 175 feet along Fremont), there will have to be rerouting of underground utilities, fencing, staging area for materials, operating of excavation and drilling equipment, trucking of spoils including contaminated materials from dumping and other fills over the 1852 shoreline, which will tie up portions of two way Mission and one way Fremont up for a year or more. The Fremont off ramp from the Bay Bridge is the major entrance to downtown every weekday morning, the option of turning on Howard is awkward because of one way streets so the blocking at Fremont and Mission will be very disruptive and will have serious effects on downtown traffic. The cost of municipal vehicles, equipment, and personnel to control traffic will have to be borne by the City.

Internal Symmetrical Plan

The internal plan (attached schematic), designed by Leslie E. Robertson Associates (LERA), Consulting Structural Engineers, designer of the World Trade Center in New York, has a total of 108 new steel jacketed piles, 54 on the Fremont side and 54 on the opposite side of the building, arranged in a symmetrical longitudinal and transverse pattern that are installed inside the basement and will perform entirely within the building's footprint. The upper portions of the 13-5/8 inch outside diameter 5/8 inch wall thickness steel rock piles are 30 feet long in and below the existing concrete mat, and inside the larger pipes are 9-5/8 inch outside diameter 1-3/8 inch wall thickness steel rock piles. The smaller piles extend through the larger piles into bedrock that is projected to be 220 feet from the bottom of the Millennium Tower mat portion of the substructure.

The entirely new pile system will provide full uniform and symmetrical vertical and lateral support for the Tower. The new lateral resistance to base shear provided by the 108 13-5/8 inch diameter piles below the mat is a much improved lateral force resisting system over the original design that relied on passive resistance against the basement wall and the prestressed concrete piles with the design assumption that the bottom of the mat substructure would remain level.

The new piles will be jacked into bedrock through holes cut through the basement slab. The jacking system uses the weight of the concrete building as a reaction. Pile locations as finally set were arranged to avoid MPE utilities, computer facilities, and fire pump house. Pile sections have threaded connections, there is no welding and annular space in the piles are grouted. The internal plan does not involve any dewatering and there is no drilling through soils. There is potential with the internal plan to at least partially return the substructure to a more level condition during the jacking process.

Costs

The cost to construct the external asymmetrical plan has been estimated by the media, quoting SGH, to be \$100M, but it will be more due to the taking of City property and services, and contingencies. The cost to construct the internal symmetrical plan has gone through joint iterations by two of the nation's major deep foundation contractors after design refinements, gravitating to less than \$90M; but the media reported much higher costs based on negotiation stances by lawyers, which led to development of the external plan.

Conclusion

The internal symmetrical plan arrests movement and restores value to the owners and the public using a completely new foundation system. Full lateral support will be provided with the internal plan, where the external plan depends in part on failed (or about to fail) now low capacity piles where adjustable input computer studies about rotated pile heads show perhaps the first major seismic event may be resisted, but with unknown residual capacities for aftershocks or subsequent earthquakes. Last week the southern part of the State had an M=6.4 event which was followed the next day by an M=7.1 event. Dewatering for the basements of buildings along Mission Street will continue as will groundwater flow under Mission Street to the Bay, but recharging if any will not continue during drought periods. The only real solution that will benefit the owners and the public in the years to come, without constant monitoring and more awful publicity for the City and engineers, is the pile support replacement using an internal symmetrical foundation plan.

The above discussed pros and cons for the two plans centered on the major problems of excavating, drilling, groundwater, and seismic resistance of eccentric partial support, but there are other adverse situations that are likely to occur with the external plan. Evidence that the piles have lost capacity and are detrimental to the system is in settlement surveys and the observed breaking and cracking of the concrete basement. As was simply stated in 1978 for the design/construction for the Southern Pacific building on Mission Street between Main and Beale Streets, "Settlement in Old Bay Mud will occur if new pressures exceed past maximum pressures." A planned 52 story building similar to Millennium Tower with a similar foundation system was considered for the 80 Natoma project, but a genuine geotechnical peer review by Prof. Charles Ladd of MIT of 6/2/04 advised Arup that the building would settle excessively (and differentially) and continue settling so SFDBI rejected the permit application. A proper peer foundation review was skipped over for the Millennium Tower (in a more difficult site than 80 Natoma), where consolidation of the Bay Mud will also continue and the building tied to failed or almost failed piles will likewise continue to settle differentially. Intuitively, and from the breaking and cracking, if the cold joint at the substructure extension does not open and if the complicated and probably uncontrollable relief system does not work, propping up two adjacent sides of the building that already has a dished foundation mat with the [partial] "Perimeter Pile Upgrade" will result in moment reversal causing the basement to break diagonally, without a way to repair and the opportunity to use the internal symmetrical foundation plan having been lost, both practically and financially.

Disclosures

Neither of the undersigned has any financial or business or other affiliation with any phase of the Millennium project or with any developer or any design professional or any contractor. However, each of the undersigned has more than 50 years experience in evaluation, design, and construction of buildings and other structures in San Francisco and the Bay Area. Both of the undersigned hold earned doctorates in Civil Engineering and other degrees from the University of California, Berkeley.

Yours truly,

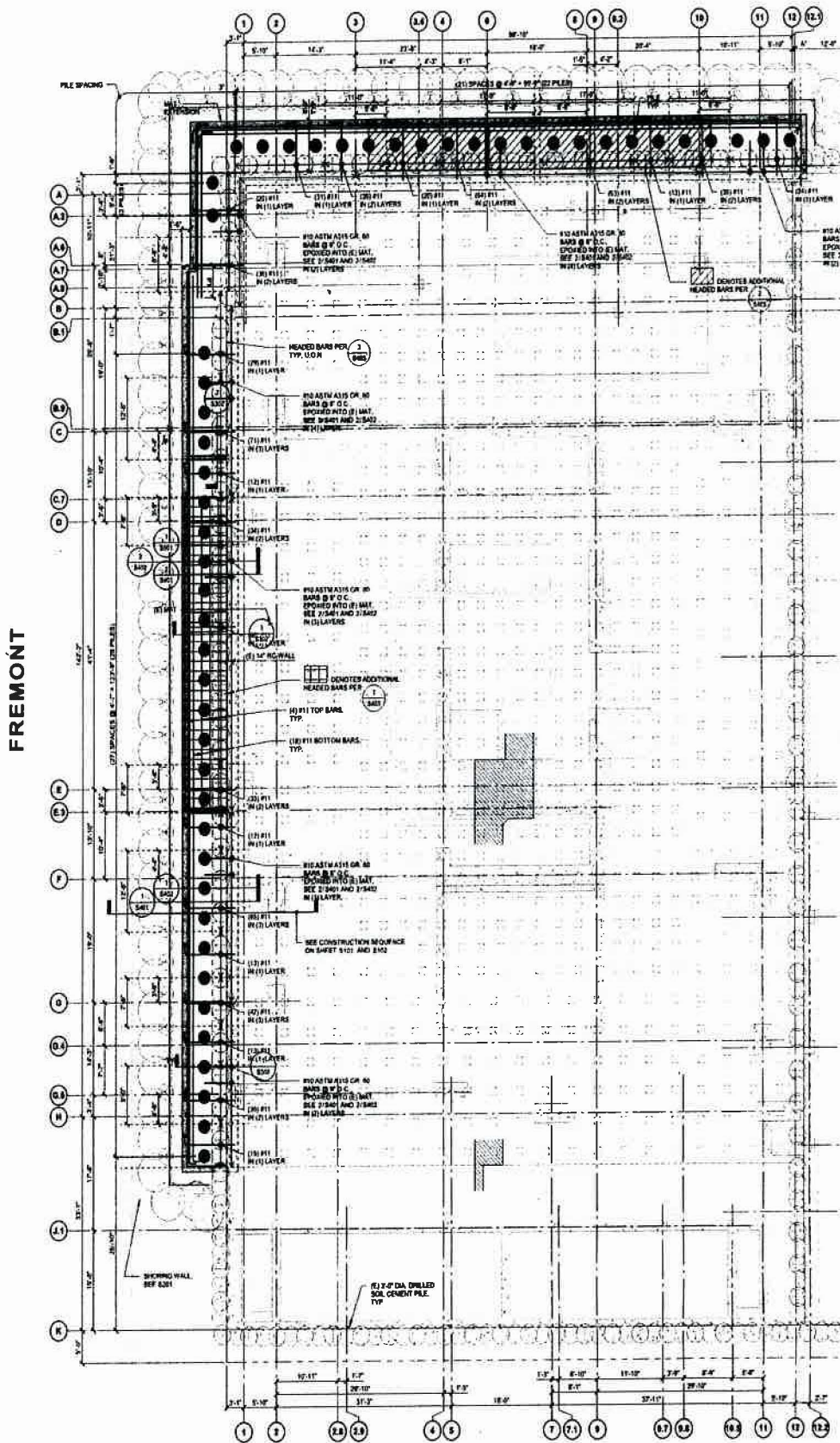
Lawrence B. Karp



Joshua B. Kardon



MISSION



EXTERNAL ASYMMETRICAL FOUNDATION PLAN

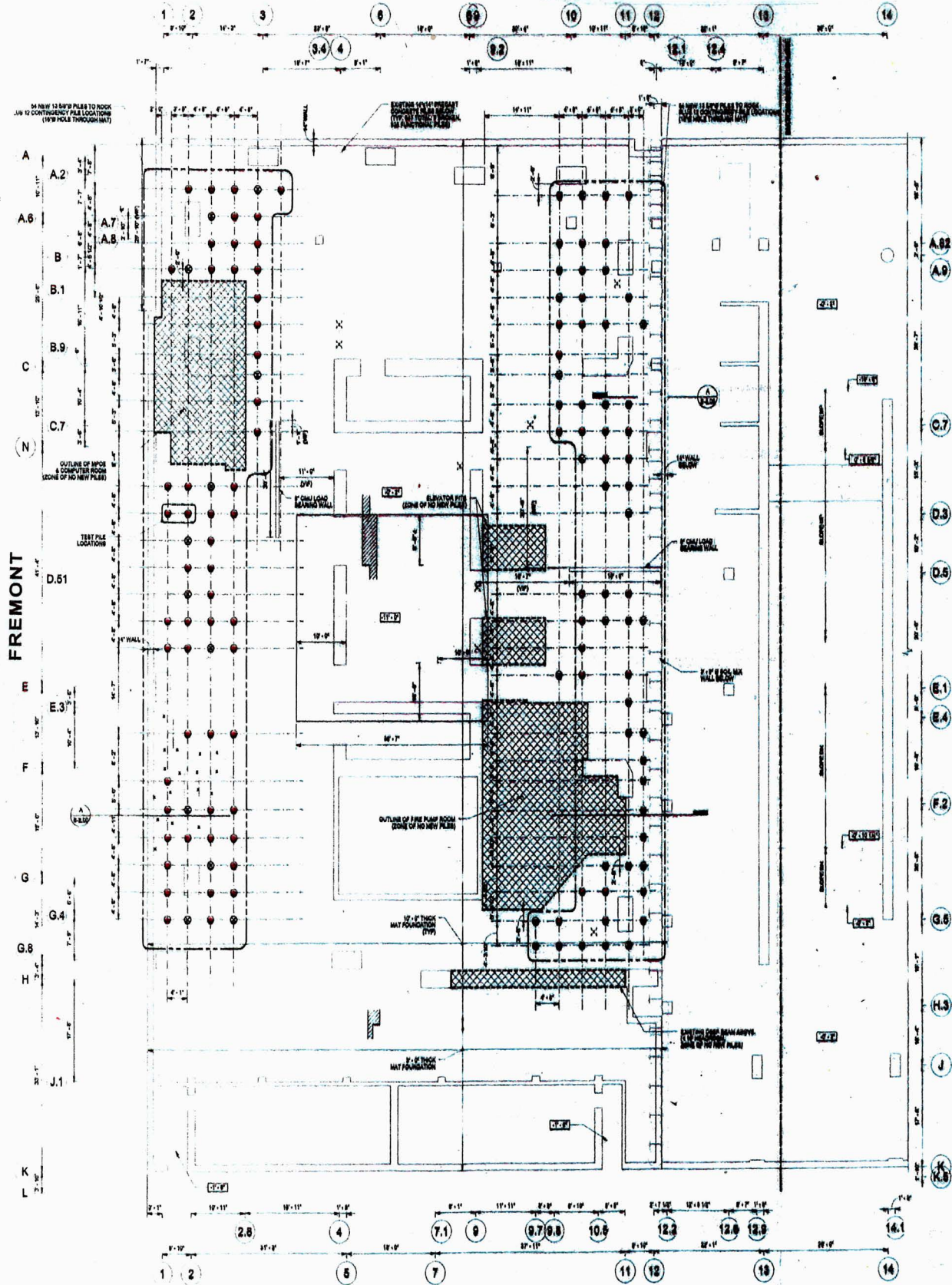
Keyplan:

MISSION STREET

MISSION



SCALE STREET



INTERNAL SYMMETRICAL FOUNDATION PLAN